Unconventional low cost simplified sewage treatment by rotating biological contactor and anaerobic (upflow) filter system

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ABSTRACT

Rotating biological contactors or biological discs and septic tank systems followed by anaerobic (upflow) filter or composite anaerobic (upflow) filter are some of the simple treatment devices relevant to semi-urban and urban fringe areas, rural areas and institutions where localised collection and treatment of sewage could be expedient. Laboratory, pilot plant and field scale studies carried out at National Environmental Engineering Research Institute, Nagpur have clearly established the feasibility of these systems and brought out the process design criteria and operational aspects. The paper brings forth briefly the salient features of the studies.

INTRODUCTION

Considering the various constraints that stand in the way of providing conventional sewerage and sewage treatment systems, the alternative is to go in for low cost simplified sewage treatment or sanitation systems, which are also hygienic and appropriate to local conditions. Low cost pour flush sanitary water seal latrine systems for individual houses is one alternative. In water logged and flooded areas, rocky areas, fissured rocks and chalk formations, these may become unsuitable from construction and underground pollution travel point of view. Adoption of alternative systems need to be given consideration. Rotating biological contactor (or biological disc), septic tanks followed by anaerobic (upflow) filter or compost anaerobic filter system followed by polishing by grass plots, are some of the unconventional simple treatment devices applicable to treat domestic sewage from small communities, institutions and urban fringe areas. The aspects regarding the feasibility of these systems, operational data and the process design criteria based on laboratory and pilot plant studies are presented briefly.

ROTATING BIOLOGICAL CONTACTOR (RBC) OR BIOLOGICAL DISC.

Experiments were conducted at NEERI, Nagpur on a prototype rotating biological contactor or biological disc followed by a secondary settling tank for treating domestic sewage. The disc chamber consists of mild steel hemispherical drum with 40 PVC circular discs of one meter diameter centrally fixed to a revolving horizontal cylindrical shaft, rotating at 5 rpm and with a clear spacing between the discs of 2.5 centimeters. The discs are half immersed in sewage and the shaft is driven by a 1.5 kW geared motor and pulley drive (Fig. 1). The treatment process is basically aerobic.

Observations (Ref. 1, 2) were made with pumped screened raw sewage, and settled municipal sewage during the period 1977-1980. Flow rate was controlled at 0.45 to 0.55 cubic meter per hour, which provided detention time of 1.25 to 1.3 hours in the disc chamber and 1 to 1.2 hours in the settling tank. The experiments were conducted with the disc chamber closed with a mild steel hemispherical cover with small perforations. Summary of the results are presented in Table-1 for RBC system treating raw sewage, without presettling.

Reduction of BODs and suspended solids to the extent of 77 to 90 per cent and 65 to 94 per cent respectively was observed at organic loading rate of 16.0 to 22.3 grams of BOD5 per square meter of disc area per day for treating screened raw sewage of BODs varying from 200 to 250 mg/l, and the power consumptions was found to be 1.0 to 1.2 kilowatt hours per kilogram of BOD5 removed. The dissolved oxygen in disc chamber was around 1 mg/l. The systems efficiency was not significantly affected due to intermittent sewage flows especially when there was no flow in the night time.

The efficiency of biological disc (Ref. 3) with reference to BOD5 for treating settled sewage improved
Table 1: Performance Data of Pilot RBC* system for treating raw sewage.

<table>
<thead>
<tr>
<th>Parameters</th>
<th>1979</th>
<th>1980</th>
</tr>
</thead>
<tbody>
<tr>
<td>Raw sewage Temp.</td>
<td>250° to 330°C</td>
<td>(Average values)</td>
</tr>
<tr>
<td>Hydraulic Loading, m³/m²/day</td>
<td>0.078</td>
<td>0.09</td>
</tr>
<tr>
<td>Organic Loading, BOD₅ gm/m²/day</td>
<td>20.7</td>
<td>16.7</td>
</tr>
<tr>
<td>BOD₅, mg/l</td>
<td>Inf. 255</td>
<td>186 31</td>
</tr>
<tr>
<td>Eff.* 45</td>
<td></td>
<td></td>
</tr>
<tr>
<td>COD, mg/l</td>
<td>Inf. 662</td>
<td>608 105</td>
</tr>
<tr>
<td>Eff.* 72</td>
<td></td>
<td></td>
</tr>
<tr>
<td>SS, mg/l</td>
<td>Inf. 466</td>
<td>156 22</td>
</tr>
<tr>
<td>Eff.* 54</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

* enclosed with perforated cover.
** after settling.

Two of such modified discs were installed in the chamber and were found to function well. Further experiments are under way using inexpensive discs consisting of web or mesh with plastic or wooden cane, netted in a circular steel or wooden rib with some spokes.

ANAEROBIC (UPFLOW FILTER)

The anaerobic filter is an upflow submerged contact filter where the wastewater is introduced from the bottom, unlike the conventional trickling filter. The microbial growth is retained on the stone media making possible higher loading rates and efficient digestion anaerobically. The anaerobic (upflow) filter was successfully tried for low strength settled and raw domestic sewage. This was also used as a secondary treatment device for treating septic tank effluent (Ref. 3, 4) in areas where dense soil conditions, high water table and limited availability of land preclude conventional soil absorption or leaching system for effluent disposal. The effluent from the anaerobic filter could be further 'polished' by passing through a grass plot, where the effluent could pick up some dissolved oxygen, resulting also in further reduction of BOD. The observations on the performance of field filter treating raw sewage, and the grass plot for polishing the filter effluent are briefly discussed.

A masonry filter box rectangular in plan of dimensions 1.6m x 1.6m x 1.4m deep (Fig. 2) was constructed in the NEERI campus, Nagpur. The filter was filled with stones of 2.0 to 2.5 cm diameter sizes to a depth of 1.2 meters supported over 15 cm thick of stone media of size 7.25 cm. The media rested on wire...
mesh. Municipal domestic sewage pumped from a municipal sewer was discharged over a V-notch and passed through the filter from the bottom. The pilot field filter can treat sewage from a small community of 40 to 60 persons. The flow was maintained at a rate of 0.42 m³/hr for a period of 8 to 10 hours daily while during the rest of the period in a day there was no flow. The detention time works out to 5 to 8 hours. The results are summarised in Table 2. Reduction of BOD₅ to the extent of 70 to 85 per cent was obtained depending on influent BOD₅ concentration while suspended solids removal was of the order of 80 to 89% and turbidity reduction of 70 to 75 per cent. Increase in ammonia concentration in the effluent was noted. The effluent was relatively clear and did not emanate any noticeable odour. The filter could work continuously for a year to 1½ years before clogging sets in. By flushing water from top and desludging from bottom, the filter can work continuously.

**Table-2: Performance Data of Anaerobic Contact Upflow Field Filter**

<table>
<thead>
<tr>
<th>Parameters</th>
<th>Influent (mg/l)</th>
<th>Effluent (mg/l)</th>
<th>Efficiency %</th>
</tr>
</thead>
<tbody>
<tr>
<td>BOD₅</td>
<td>210</td>
<td>43</td>
<td>79.5</td>
</tr>
<tr>
<td>S.S</td>
<td>(112-310)</td>
<td>(12-82)</td>
<td></td>
</tr>
<tr>
<td>NH₃-N</td>
<td>41</td>
<td>52</td>
<td>-</td>
</tr>
<tr>
<td>pH</td>
<td>7.3</td>
<td>7.4</td>
<td>-</td>
</tr>
</tbody>
</table>

**Table-3: Overland Flow Grass Filtration of Anaerobic Filter Effluent.**

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Influent Raw sewage</th>
<th>Anaerobic filter Effluent</th>
<th>Final Effluent after grass plot</th>
</tr>
</thead>
<tbody>
<tr>
<td>BO₅</td>
<td>230</td>
<td>74</td>
<td>49</td>
</tr>
<tr>
<td>S.S</td>
<td>(120-310)</td>
<td>(35-132)</td>
<td>(14-105)</td>
</tr>
<tr>
<td>D.Q.</td>
<td>212</td>
<td>92</td>
<td>55</td>
</tr>
<tr>
<td></td>
<td>(98-350)</td>
<td>(35-132)</td>
<td>(14-87)</td>
</tr>
</tbody>
</table>

**GRASS PLOT**

Overland flow through grass plots was used for 'polishing' of the anaerobic filter effluent. Dumgrass (Cynodon sp.) was grown in small plots of size 3m x 1.5m and 6m x 1m (Fig. 2). The filter effluent was allowed to flow over these grass plots every alternate day. There was build up of dissolved oxygen (1 to 2 mg/l) and further reduction in BOD as the effluent flowed out of the grass plot (vide Table 3).
CONCLUSIONS.

Rotating Biological Contactor (RBC) and Anaerobic filter systems are simple to construct, and operate at low head loss and are relatively free from odour nuisance. RBC system is compact, has simple mechanical gadgets and power consumption is moderate, while the anaerobic filter does not have any mechanical gadgets. Both can function satisfactorily when the flow of sewage is intermittent.

RBC can be operated with raw sewage instead of settled sewage, thereby eliminating presettling tank. Even though the efficiency is lower with raw sewage compared to settled sewage, the performance can be improved by increasing the detention time of disc and settling chamber of pilot plant and decreasing the organic loading rate to get an efficiency more than 90 per cent.

The anaerobic filter can function as a composite secondary treatment device in which case the sewage should be free from grit and preferably partly homogenised. The filter can also be used as a secondary treatment device for treating effluent from septic tanks. Overland grass filtration treatment of filter effluent will be an useful adjunct for further 'polishing' the effluent from filter or RBC. The RBC system, septic tank followed by anaerobic upflow filter and grass plot, or composite anaerobic upflow filter followed by grass plot are suitable for treating sanitary sewage from institutions, small communities and individual houses. Based on the studies so far, the process design criteria can be briefly stated as follows:

**RBC** for treating raw sewage.

Organic Loading rate to disc chamber: 16 to 20 gms BOD$_5$/m$^2$/of disc area/day.

Overall BOD removal efficiency: 90%

Detention time in disc chamber: 1.5 hrs.

RPM of disc: 5

Temp. of sewage: 23 to 32$^\circ$C.

Influent BOD concentration: 200 to 300 mg/l

**Anaerobic Filter**

Media (Gravel, broken stone)

Size of media: 1.9 cms to 2.5 cms.

Depth of media: 115 cms to 125 cms.

Hydraulic Loading rate: 3.4 m$^3$/m$^2$/day.

Temp. of sewage: 23$^\circ$C to 33$^\circ$C.

BOD removal efficiency: 70 to 80% for influent BOD concentration of 110 mg/l to 300 mg/l.

Grass Plot: Hydraulic Loading 0.8 to 1.5 m$^3$/day/sq. meter of area minimum 2 plots.

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